



# Ourston Roundabout Engineering

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July 16, 2010

Maine Bureau of Transportation Systems Planning  
16 State House Station  
Augusta, ME 04333-0016

Attention: Mr. Darryl Belz, P.E.

Dear Darryl:

**Re: Roundabout Operational Analysis and Design Services  
Proposed Roundabout at Civic Center Drive/Old Belgrade Rd/Bog Rd  
Augusta, Maine  
Ourston Project Number: 09909**

We have completed an operational analysis and conceptual design for a proposed roundabout to be located at the intersection of Civic Center Drive, Old Belgrade Road, and Bog Road in Augusta, Maine. The projected peak hour traffic volumes for the build year 2028 Midday and PM peak hours, developed by Gorrill-Palmer Consulting Engineers, were used in the traffic analysis. The volumes were taken from pages 19 and 20 of the traffic report, shown in Figures A1 and A2.

At this stage of the investigation into alternative intersection control, there is sufficient evidence to recommend consideration of a roundabout in this location to accommodate traffic increase and associated roadway network improvements. We have provided an initial screening of the intersection quantifying the performance characteristics of a roundabout based on operations and property impacts.

## **Roundabout Capacity Analysis**

The traffic operations for the intersection were analyzed using RODEL 1.9.7 roundabout design and capacity analysis software. The 50<sup>th</sup> percentile confidence level (CL) was used in the RODEL capacity analysis to represent the most probable capacity of the roundabout, and to be consistent with confidence level inherent in typical signalized and unsignalized capacity analysis methodologies. Similarly, average delay, as opposed to maximum delay, was used to be consistent with signalized and unsignalized methodologies. We have also performed capacity analysis at an 85 percent confidence level to determine if the forecasted level of service and average delay is predicted to be in the sensitive area of the delay curve.

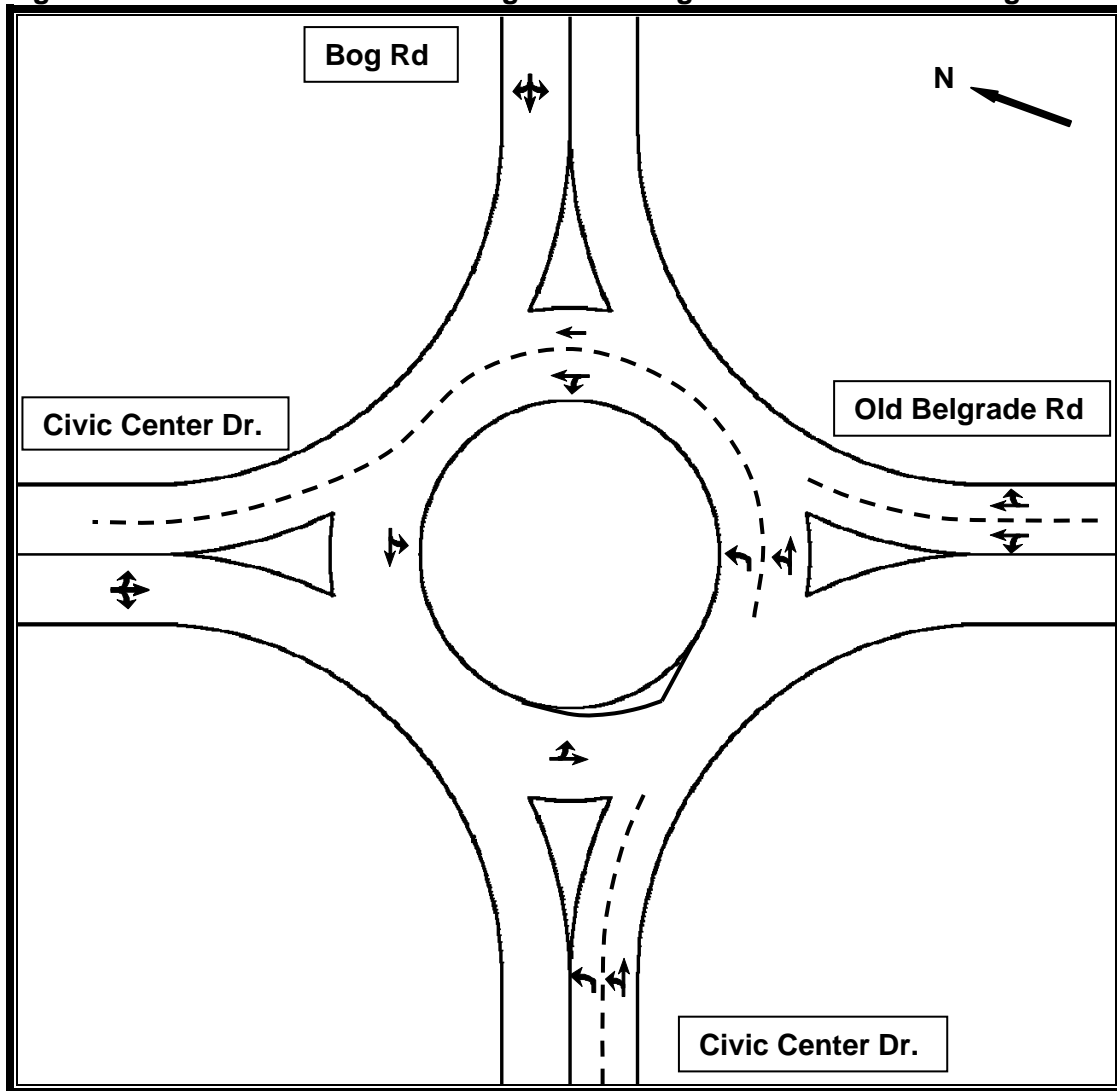
A single lane roundabout configuration was considered with single lane entries on all legs. The capacity analysis indicates that the single lane roundabout would perform acceptably up to **122% of the Midday peak volume** and up to **80% of the PM peak volume**.

To provide acceptable operations for the build year 2028 volumes, a multi-lane roundabout will be required. The layout of the intersection would be a multi-lane roundabout with two lane entries on Northbound Civic Center Drive and Northwestbound Old Belgrade Road and single lane entries on Southbound Civic Center Drive and Bog Road. Although the capacity prediction does not require it, a partial right-turn bypass lane could be constructed on Southbound Civic Center Drive to



accommodate the high volume of traffic making this movement. A sketch of the required roundabout lane configuration requirements is shown in Figure 1.

**Figure 1: Civic Center Drive/Old Belgrade Rd/Bog Rd – Multi-Lane Configuration**



The RODEL capacity analysis data is documented in Figures A3 through A6 for the 2028 Midday and PM volumes.

The lane configuration's levels of service and delay for the overall intersection and each approach are listed in Table 1.



**Table 1: Civic Center Drive/Old Belgrade Rd/Bog Rd – 2028 Volumes**

Civic Center Drive / Old Belgrade Road / Bog Road - 2028 Volumes - Capacity Analysis Summary											
		Intersection		Average Delay By Approach							
				Civic Center SB		Civic Center NB		NWB Old Belgrade		SWB Bog Rd	
		Level of Service	Average Delay	Level of Service	Average Delay	Level of Service	Average Delay	Level of Service	Average Delay	Level of Service	Average Delay
Midday	50%	A	6.2	B	11.4	A	3.3	A	2.9	A	6.2
	85%	B	14.1	D	33.4	A	4.0	A	3.5	A	9.6
PM	50%	A	6.0	A	5.6	A	6.3	A	5.1	A	8.8
	85%	A	9.3	A	8.1	A	9.9	A	7.2	C	18.1
LOS Source: 2000 Highway Capacity Manual - Unsignalized Intersections										Delay in Seconds	

An operational sensitivity analysis was conducted to estimate the point of unacceptable operations for the proposed lane configuration. The 2028 traffic volumes were consistently increased to a point where any leg of the roundabout would reach a Level of Service of E (35 seconds of delay) at 50% CL. The results show that the multi-lane roundabout would operate acceptably up to **122% of the Midday peak volume** and up to **130% of the PM peak volume**.

A summary of the operational analysis is also shown in Figure A7.

### **Roundabout Conceptual Design**

The conceptual roundabout design has an inscribed circle diameter (ICD) of 165 feet. A partial right-turn bypass lane is an optional element for Southbound Civic Center Drive to accommodate the high volume of traffic making this movement. Two alternatives have been developed showing two different circle locations. Each alternative has a set of challenges to maintain access to the adjacent properties; we have shown the two options to demonstrate the impacts of changes in the circle location. Shifting the circle location and the associated approach alignments influences the ability to maintain accessibility of the surrounding land parcels. A cost estimate detailing the costs of property acquisition and access road construction should be completed at the next stage of the design process to determine the most cost effective circle location. After a general cost estimate has been conducted, a more refined approach to the roundabout design optimization can be completed. The conceptual design of the roundabout is shown in Figures B1 and B2.

The roundabout concepts were developed to accommodate large trucks (WB-67), provide adequate sight distance and vision outlook, and provide natural vehicle paths. The entry geometry of the roundabout was designed to deflect traffic in order to slow speeds below 27 miles per hour and reinforce the yielding process. Entry path deflection affects the frequency and severity of collisions between entering and circulating traffic. It is apportioned based on the combination and proportion of the conflicting streams of traffic.

Additional effort to optimize this design will be required for detailed design. The possible modifications include shifts in circle locations, alignment adjustments, development of optimal entry/exit paths, and property access optimization. These geometric considerations affect the safety and capacity performance of roundabouts.



Maine Bureau of Transportation Systems Planning  
Mr. Darryl Belz, P.E.  
July 16, 2010  
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Yours truly,

**OURSTON ROUNDABOUT ENGINEERING INC.**

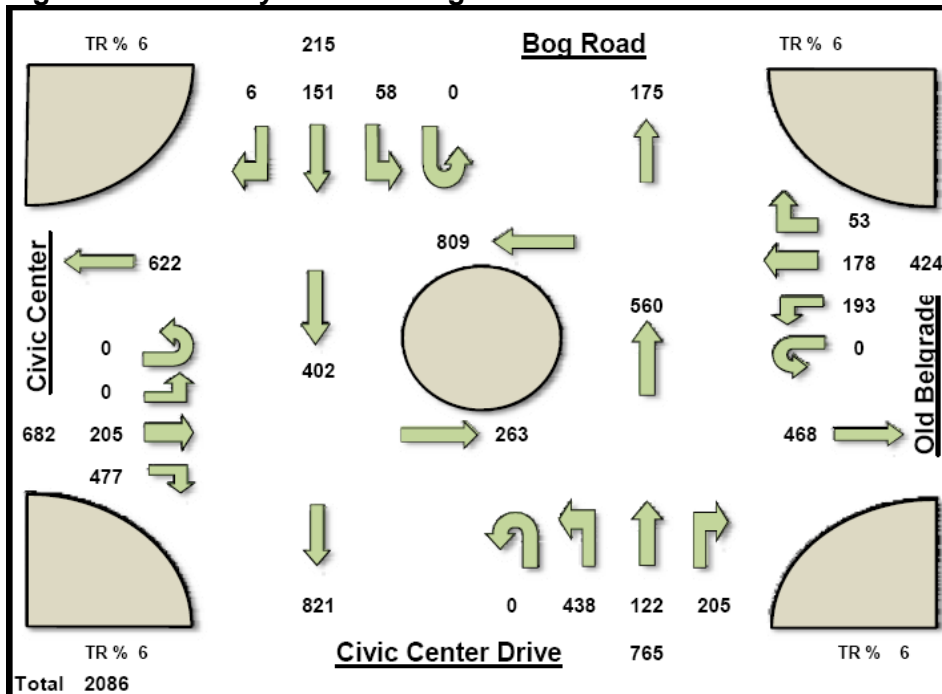
A handwritten signature in black ink, reading "Troy Pankratz".

Troy Pankratz, P.E.  
Project Manager

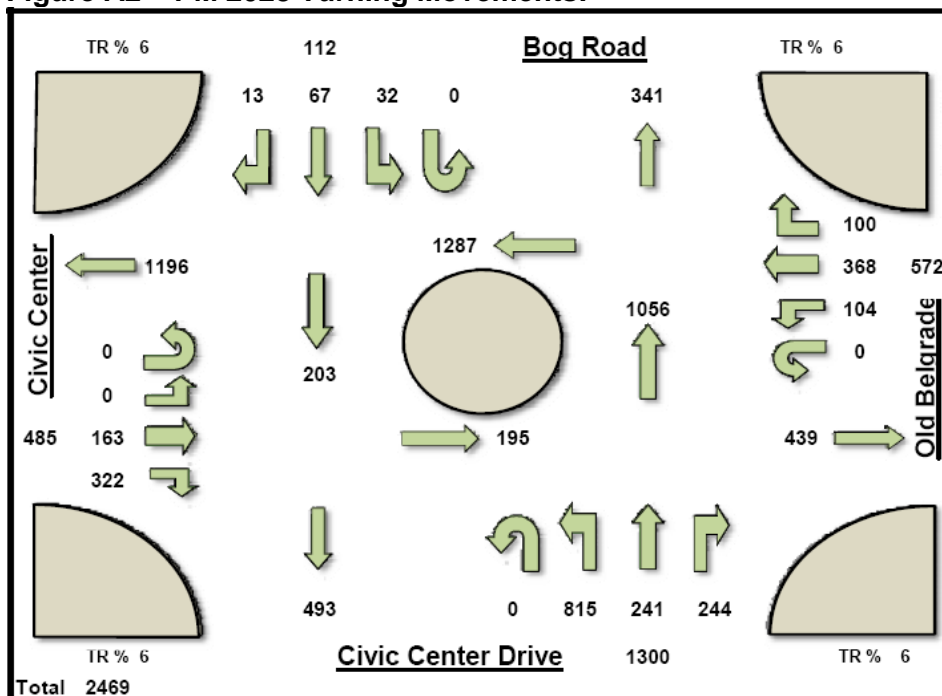
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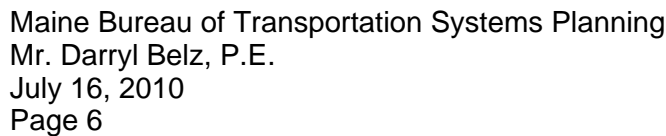



**Figure A1 – Midday 2028 Turning Movements:**



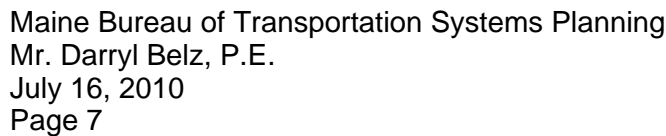
**Figure A2 – PM 2028 Turning Movements:**






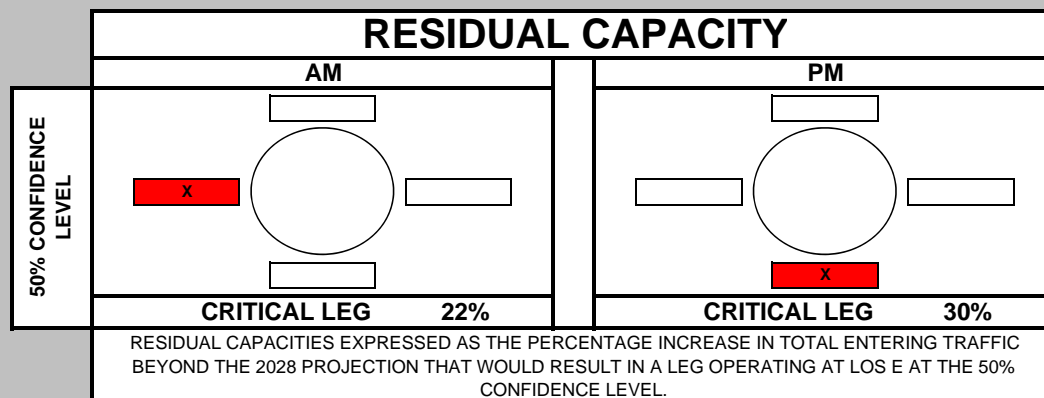
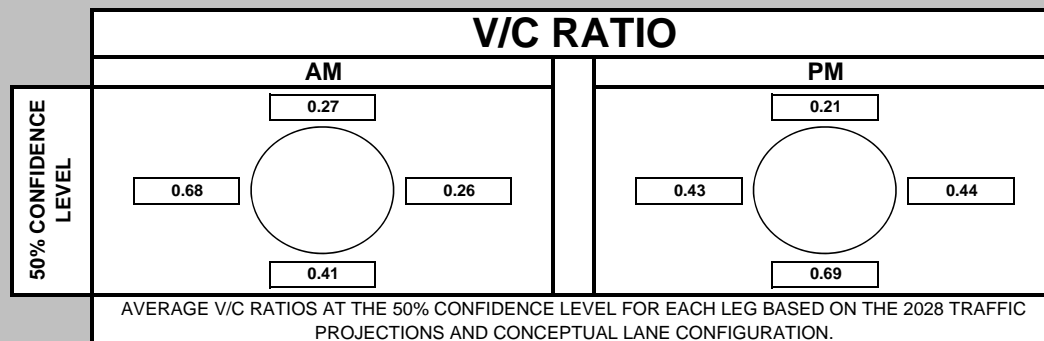
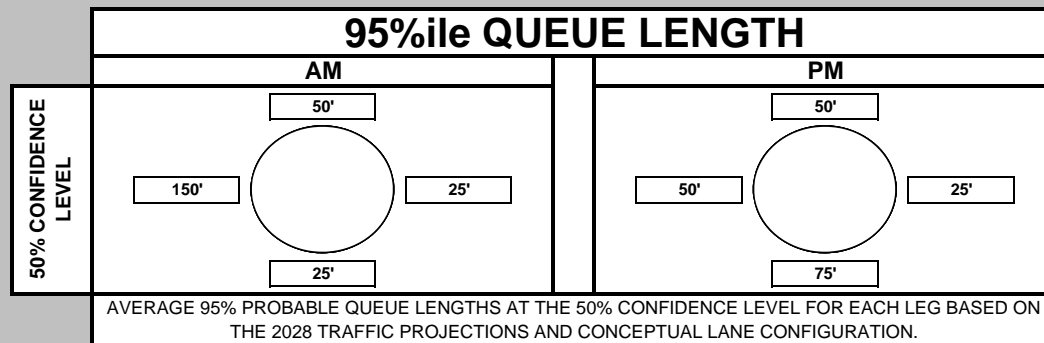
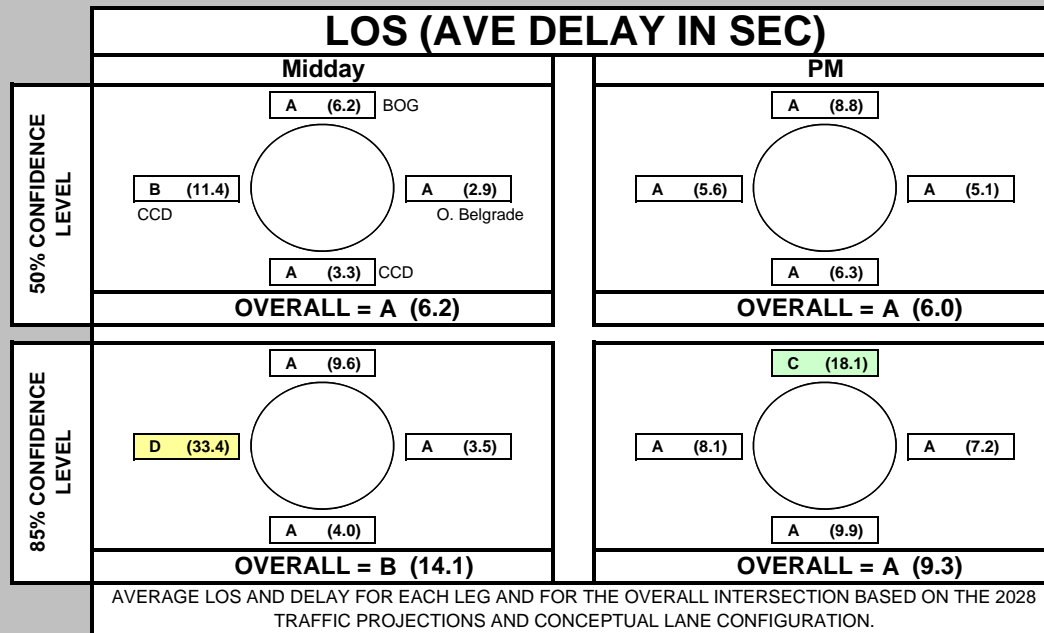
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29:6:10					Old Belgrade / Bog Road 2028											
E	<m>	4.25	8.00	8.00	4.25	TIME PERIOD					min	90				
L'	<m>	10.00	40.00	40.00	10.00	TIME SLICE					min	15				
U	<m>	3.65	3.65	3.65	3.65	RESULTS PERIOD					min	15	75			
RAD	<m>	20.00	20.00	20.00	20.00	TIME COST					\$/hr	15.00				
PHI	<d>	20.00	20.00	20.00	20.00	FLOW PERIOD					min	15	75			
DIA	<m>	50.00	50.00	50.00	50.00	FLOW TYPE					pcu/veh	VEH				
GRAD	SEP	0	0	0	0	FLOW PEAK					am/op/pm	OP				
LEG NAME		PCU	TURNS <1st exit, 2nd..U>			FLOF	CL	FLOW RATIO				FLOW TIME				
SB CCD		1.06	477	205	0	0	1.00	50	0.75	1.125	0.75	15	45	75		
NB CCD		1.06	205	122	438	0	1.00	50	0.75	1.125	0.75	15	45	75		
NWB OldBel		1.06	53	178	193	0	1.00	50	0.75	1.125	0.75	15	45	75		
SWB Bog Rd		1.06	6	151	58	0	1.00	50	0.75	1.125	0.75	15	45	75		
MODE 2																
FLOW		veh	682	765	424	215	AVEDEL					s	6.2			
CAPACITY		veh	1010	1849	1639	789	LOS					SIG	A			
AVE DELAY		secs	11.4	3.3	2.9	6.2	LOS					UNSIG	A			
MAX DELAY		secs	17.8	4.4	3.9	8.6										
AVE QUEUE		veh	2.2	0.7	0.4	0.4	VEHIC HRS						3.6			
MAX QUEUE		veh	3.1	0.8	0.4	0.5	COST					\$	54			
F1mode		F2direct	F3peak	CtrlF3rev	F4fact	F6stats	F8econ	F9prnt	F10run	Esc						

29:6:10 Old Belgrade / Bog Road 2028 28														
E	<m>	4.25	8.00	8.00	4.25	TIME PERIOD				min	90			
L'	<m>	10.00	40.00	40.00	10.00	TIME SLICE				min	15			
U	<m>	3.65	3.65	3.65	3.65	RESULTS PERIOD				min	15 75			
RAD	<m>	20.00	20.00	20.00	20.00	TIME COST				\$/hr	15.00			
PHI	<d>	20.00	20.00	20.00	20.00	FLOW PERIOD				min	15 75			
DIA	<m>	50.00	50.00	50.00	50.00	FLOW TYPE				pcu/veh	VEH			
GRAD	SEP	0	0	0	0	FLOW PEAK				am/op/pm	OP			
LEG NAME		PCU	TURNS <1st exit, 2nd..U>			FLOF	CL	FLOW RATIO				FLOW TIME		
SB CCD		1.06	477	205	0 0	1.00	85	0.75	1.125	0.75	15	45	75	
NB CCD		1.06	205	122	438 0	1.00	85	0.75	1.125	0.75	15	45	75	
NWB OldBel		1.06	53	178	193 0	1.00	85	0.75	1.125	0.75	15	45	75	
SWB Bog Rd		1.06	6	151	58 0	1.00	85	0.75	1.125	0.75	15	45	75	
MODE 2														
FLOW	veh	682	765	424	215	AVEDEL				s	14.1			
CAPACITY	veh	815	1654	1444	594	LOS				SIG	B			
AVE DELAY	secs	33.4	4.0	3.5	9.6	LOS				UNSIG	B			
MAX DELAY	secs	63.2	5.5	4.7	13.9	VEHIC HRS					8.2			
AVE QUEUE	veh	6.5	0.9	0.4	0.6	COST				\$	122			
MAX QUEUE	veh	11.4	1.1	0.5	0.7									
F1mode	F2direct	F3peak	CtrlF3rev	F4fact	F6stats	F8econ	F9prnt	F10run	Esc					

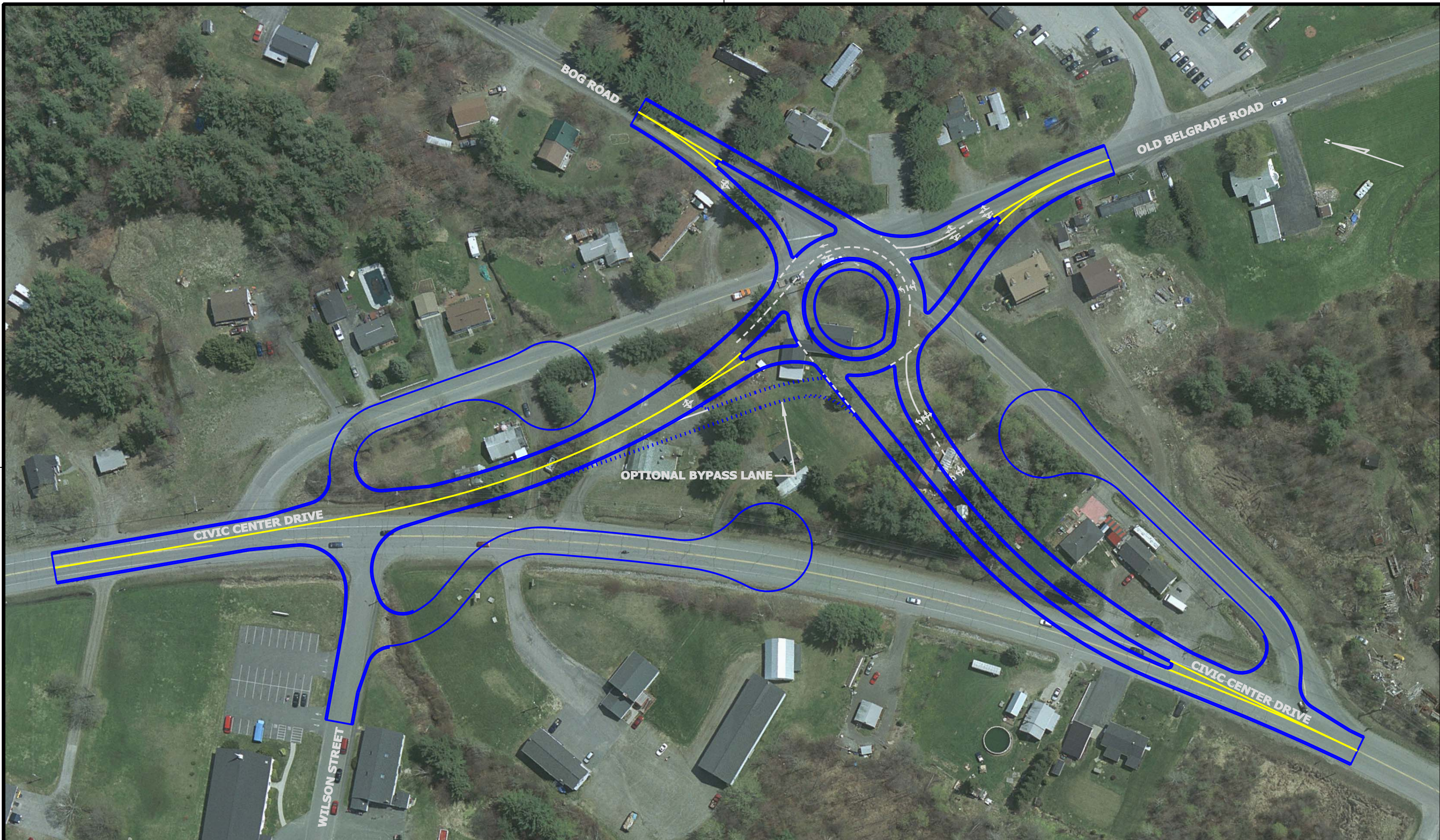


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L'	<m>	10.00	40.00	40.00	10.00	TIME SLICE					min	15		
U	<m>	3.65	3.65	3.65	3.65	RESULTS PERIOD					min	15	75	
RAD	<m>	20.00	20.00	20.00	20.00	TIME COST					\$/hr	15.00		
PHI	<d>	20.00	20.00	20.00	20.00	FLOW PERIOD					min	15	75	
DIA	<m>	50.00	50.00	50.00	50.00	FLOW TYPE					pcu/veh	VEH		
GRAD	SEP	0	0	0	0	FLOW PEAK					am/op/pm	PM		
LEG NAME		PCU	TURNS <1st exit, 2nd..U>			FLOF	CL	FLOW RATIO				FLOW TIME		
SB CCD		1.06	322	163	0	0	1.00	50	0.75	1.125	0.75	15	45	75
NB CCD		1.06	244	241	815	0	1.00	50	0.75	1.125	0.75	15	45	75
NWB OldBel		1.06	100	368	104	0	1.00	50	0.75	1.125	0.75	15	45	75
SWB Bog Rd		1.06	13	67	32	0	1.00	50	0.75	1.125	0.75	15	45	75
MODE 2														
FLOW	veh	485	1300	572	112						AUEDEL	s	6.0	
CAPACITY	veh	1118	1897	1290	530						LOS	SIG	A	
AUE DELAY	secs	5.6	6.3	5.1	8.8						LOS	UNSIG	A	
MAX DELAY	secs	7.6	9.5	7.5	12.9									
AUE QUEUE	veh	0.8	2.3	0.8	0.3						VEHIC HRS	4.1		
MAX QUEUE	veh	0.9	3.1	1.1	0.4						COST	\$	61	
F1mode	F2direct	F3peak	CtrlF3rev	F4fact	F6stats	F8econ	F9prnt	F10run	Esc					

MODEL											
29:6:10				Old Belgrade / Bog Road 2028						30	
E	<m>	4.25	8.00	8.00	4.25	TIME PERIOD		min	90		
L'	<n>	10.00	40.00	40.00	10.00	TIME SLICE		min	15		
V	<n>	3.65	3.65	3.65	3.65	RESULTS PERIOD		min	15	75	
RAD	<m>	20.00	20.00	20.00	20.00	TIME COST		\$/hr	15.00		
PHI	<d>	20.00	20.00	20.00	20.00	FLOW PERIOD		min	15	75	
DIA	<m>	50.00	50.00	50.00	50.00	FLOW TYPE		pcu/veh	VEH		
GRAD SEP		0	0	0	0	FLOW PEAK		am/op/pm	PM		
<b>LEG NAME PCU TURNS &lt;1st exit, 2nd..U&gt; FLOF CL FLOW RATIO FLOW TIME</b>											
SB CCD	1.06	322	163	0	0	1.00	85	0.75	1.125	0.75	15 45 75
NB CCD	1.06	244	241	815	0	1.00	85	0.75	1.125	0.75	15 45 75
NWB OldBel	1.06	100	368	104	0	1.00	85	0.75	1.125	0.75	15 45 75
SWB Bog Rd	1.06	13	67	32	0	1.00	85	0.75	1.125	0.75	15 45 75
<b>MODE 2</b>											
FLOW	veh	485	1300	572	112	AVEDEL s		9.3			
CAPACITY	veh	923	1701	1095	334	LOS SIG		A			
AUE DELAY	secs	8.1	9.9	7.2	18.1	LOS UNSIG		A			
MAX DELAY	secs	11.5	16.2	11.1	29.8						
AUE QUEUE	veh	1.1	3.7	1.2	0.6	VEHIC HRS		6.4			
MAX QUEUE	veh	1.4	5.3	1.6	0.9	COST \$		95			
F1mode	F2direct	F3peak	CtrlF3rev	F4fact	F6stats	F8econ	F9prnt	F10run	Esc		







Augusta North - Augusta, Maine  
Civic Center Drive / Old Belgrade Road / Bog Road  
Roundabout Conceptual Design - Alternative #1



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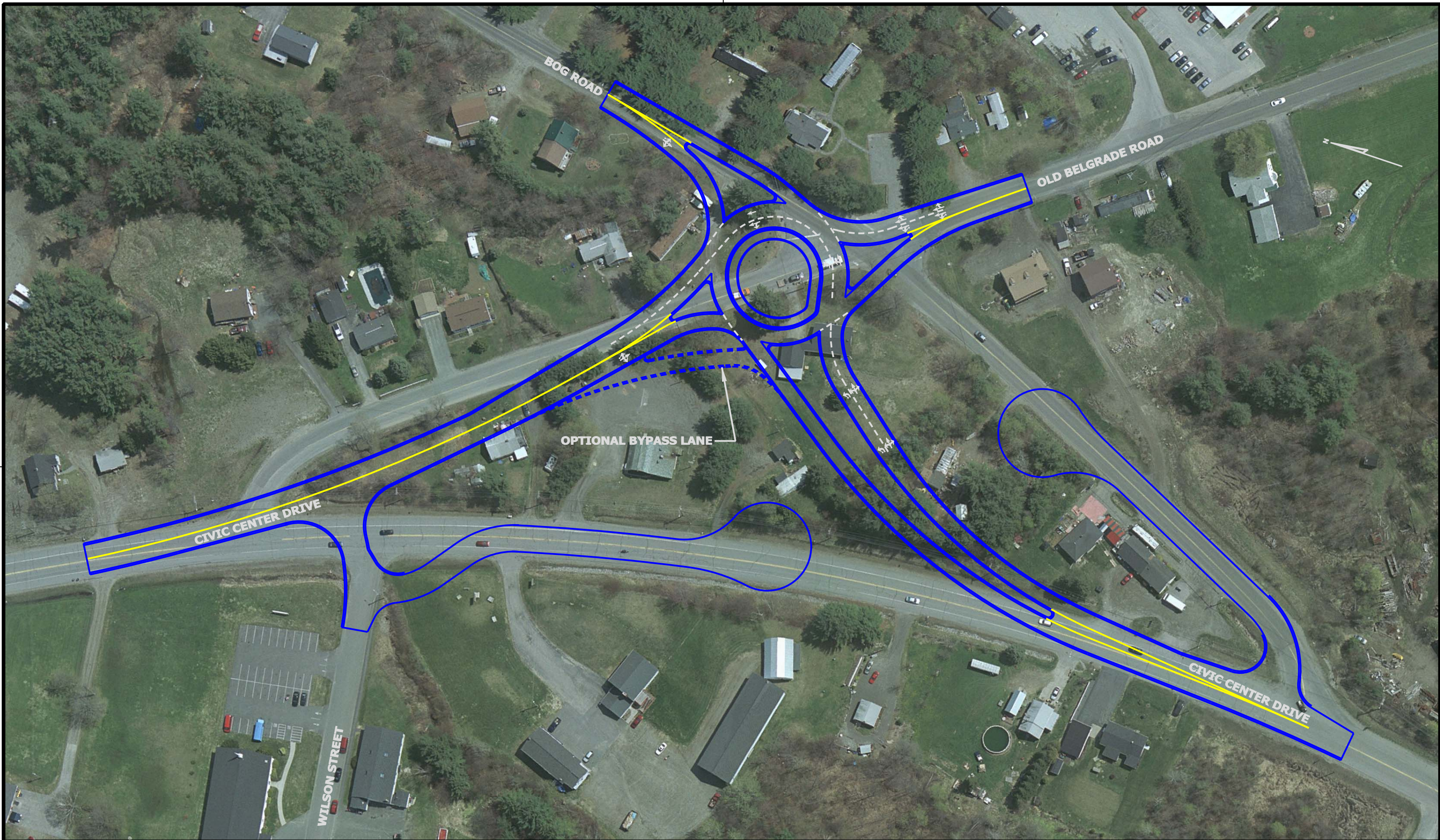
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SCALE  
1"= 100

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FIGURE No.  
B1





Augusta North - Augusta, Maine  
Civic Center Drive / Old Belgrade Road / Bog Road  
Roundabout Conceptual Design - Alternative #2



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PROJECT No.  
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SCALE  
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FIGURE No.  
B2